

**GEOTECHNICAL INVESTIGATION REPORT
NEW GANARASKA FOREST CENTRE
KIRBY, GANARASKA REGION, ONTARIO
PROJECT NO. 06-G-184 (G020398-A1)**

Prepared for:

The Ganaraska Region Conservation Authority
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GEO-LOGIC INC.

Geotechnical Engineers
and Hydrogeologists

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JANUARY, 2007

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**GEOTECHNICAL INVESTIGATION REPORT
NEW GANARASKA FOREST CENTRE
KIRBY, GANARASKA REGION, ONTARIO
PROJECT NO. 06-G-184 (G020398-A1)**

1.0 INTRODUCTION

This report presents the results of a geotechnical investigation performed for the proposed construction of a new Ganaraska Forest Centre at the existing Ganaraska Forest Centre site on Cold Springs Camp Road near Kirby, in the Ganaraska Region of Ontario. Geo-Logic Inc. (Geo-Logic) was retained by the Ganaraska Regional Conservation Authority (GRCA) to complete this geotechnical investigation in accordance with Geo-Logic's proposal dated November 9, 2006. In addition to the scope outlined in Geo-Logic's proposal, GRCA requested that additional work be conducted in order to provide soil parameters for septic system design. The work conducted for this investigation was carried out under the authorization of Mr. Mark Peacock, P.Eng., representing GRCA.

A site plan was provided to Geo-Logic Inc. that illustrated the layout and location of the proposed Forest Centre building. It is Geo-Logic Inc.'s understanding that the new structure will not include a basement area. In addition, based on information provided by Mr. Peacock, it is Geo-Logic's understanding that a septic system shall be constructed approximately south/southwest of the proposed building location.

2.0 PURPOSE AND SCOPE

The purpose of this geotechnical investigation is to define the subsurface soil and groundwater conditions at the test pit locations, and to develop geotechnical engineering conclusions and recommendations regarding earthwork construction, backfill, dewatering, slab-on-grade and foundation design.

Please note that this work, and the contents of this report, must in no way be construed as an environmental opinion of this site. Should such an opinion be desired, it is recommended that an Environmental Site Assessment (ESA) be conducted.

The following scope of work was performed in order to accomplish the foregoing purposes.

1. Clearance of underground services prior to advancing test pits. The test pits were located as illustrated on the attached Test Pit Location Plan (Plate 1).
2. The subsurface conditions were explored by excavating, sampling and logging a total of eight (8) test pits.
3. Laboratory analyses of materials encountered was performed, including grain size distribution analyses and moisture content tests.
4. Geotechnical engineering analysis of acquired field and laboratory data, and preparation of a geotechnical investigation report outlining our relevant findings, conclusions, and recommendations.

3.0 FIELD AND LABORATORY PROCEDURES

A field investigation was conducted under the supervision of Geo-Logic staff on December 15, 2006. The test pits were located as shown on the attached Test Pit Location Plan (Plate 1). Geo-Logic Inc.'s fieldwork consisted of subsurface exploration by means of excavating and sampling a total of eight (8) exploratory test pits to depths ranging from about 1.5 to 3.0 metres below existing grade (mbeg). A detailed log of each test pit was maintained (see Appendix A). Representative samples of the materials encountered in the test pits were obtained and returned to the laboratory for detailed assessment.

The test pits were advanced using a rubber-tired backhoe. Representative, disturbed samples of the strata penetrated were obtained either from the backhoe's bucket, or directly from the test pit walls.

Soil samples obtained from the test pits were inspected in the field immediately upon retrieval for type, texture, and colour. Groundwater level measurements were obtained from the open test pits immediately following drilling operations. A percolation test was conducted in the native, sandy soil within one open test pit, in the proposed area of the new septic system. The test pits were backfilled following completion of the fieldwork. All samples were sealed in clean plastic containers and transported to the Geo-Logic laboratory for further visual-tactile examination, and to select appropriate samples for laboratory analysis.

Laboratory testing was completed on selected soil samples, and consisted of moisture content tests on all recovered samples and gradation analyses on two (2) representative soil samples. The analytical results of the moisture content and gradation tests are incorporated into the attached logs in Appendix A. The results of the gradation tests are also presented graphically in Appendix B.

4.0 SITE LOCATION AND SURFACE CONDITIONS

The investigated site is located on Cold Springs Camp Road, approximately between Kendal and Elizabethville. A Ganaraska Forest facility presently exists, in the form of a number of buildings and parking areas. One of these buildings, and an existing weather-monitoring tower, are indicated on the Test Pit Location Plan (Plate 1) for reference purposes.

The investigated site is relatively flat, and consists predominantly of grass with some sporadic trees. No pooled or standing water was observed at the surface of the site.

5.0 SUBSURFACE CONDITIONS

5.1 GENERAL

Details of the subsurface conditions encountered at the site are presented graphically on the logs, (see Appendix A). It should be noted that the boundaries between the strata have been inferred from test pit observations and non-continuous samples. They generally represent a transition from one soil type to another, and should not be inferred to represent an exact plane of geological change. Further, conditions will vary between and beyond the test pits. Following is a summarized account of the subsurface conditions encountered in the test pits.

The test pits generally encountered a surficial layer of topsoil over loam underlain by native soils consisting of sand and gravel. Groundwater was not observed in any of the open test pits. The following sections describe the major soil strata and subsurface conditions encountered during this investigation in more detail.

5.2 TOPSOIL AND LOAM

A layer of surficial topsoil was observed in all the test pits. This material ranged in thickness from approximately 0.2 to 0.3m. A layer of reddish brown silty loam was encountered immediately beneath the topsoil in all but one of the test pits. The loam contained root fibres, and extended to depths ranging between approximately 0.3 to 0.5 mbeg.

The topsoil was observed to be in a damp, loose state, with a silty, occasionally sandy texture and contained a relatively high organic content. As such, they are expected to be devoid of any structural engineering properties.

5.3 NATIVE INORGANIC SOILS

Native, inorganic soils were observed immediately beneath the topsoil and loam in all eight test pits. These soils primarily consisted of sand and gravel, with typically trace amounts of silt. Occasional pockets of finer-grained soils (containing higher amounts of silt) were observed. Also, occasional cobbles and boulders were observed within the sand and gravel soil. This soil was generally in a compact to dense in-situ state, and exhibited a brown to grey colouration, and occasional layering. These soils extended to the full depth of the investigation in each test pit.

Moisture content tests conducted on samples of the sand and gravel soils yielded values ranging from about 5 to 12 % moisture by weight. Grain size gradation analyses conducted on samples of these soils suggest the following compositions:

- silty sand: 0 % gravel, 69 % sand, and 31 % silt and clay-sized particles; and
- gravel with sand: 86 % gravel, 11 % sand, and 3 % silt and clay-sized particles.

The results of the laboratory testing, combined with visual-tactile examination of the soils indicates that they are expected to exhibit a low frost susceptibility, and at the time of Geo-Logic's fieldwork appeared to be approximately at or near their optimal moisture content for compaction purposes.

5.4 GROUNDWATER

Groundwater conditions were obtained directly from the open test pits during and upon completion of excavation operations. All eight (8) test pits remained dry throughout their excavation.

Please note that groundwater levels are generally transient and tend to fluctuate with the seasons, periods of precipitation, and temperature.

6.0 DISCUSSION AND RECOMMENDATIONS

6.1 GENERAL

Supporting data upon which our recommendations are based have been presented in the foregoing sections of this report. The following recommendations are governed by the physical properties of the subsurface materials that were encountered at the site, and assumes that they are representative of the overall site conditions. *It should be noted that these conclusions and recommendations are intended for use by the designers only.* Contractors bidding on or undertaking any work at the site should examine the factual results of the investigation, satisfy themselves as to the adequacy of the information for construction, and make their own interpretation of this factual data as it affects their proposed construction techniques, equipment capabilities, costs, sequencing, and the like. Comments, techniques, or recommendations pertaining to construction should not be construed as instructions to the contractor.

Based on the information gathered during this investigation, this site's subsurface stratigraphy generally consists of a surficial layer of topsoil over loam underlain by native soils consisting of sand and gravel. Groundwater was not observed in any of the open test pits.

Details regarding our conclusions and recommendations are outlined in the following sections.

6.2 SITE PREPARATION, EXCAVATION, AND BACKFILL

It is recommended that any and all vegetation, fill, organics, and organic-bearing soils be stripped and removed from the building envelope, parking and access areas prior to commencing earthwork construction. The subexcavated surface must be proof rolled and/or approved by a member of Geo-Logic prior to placement of fill or foundations.

Due to its gradation, the excavated native, inorganic soils are generally expected to be suitable for reuse as backfill for trenches. In addition, the native sand and gravel soils may be suitable for reuse as backfill to foundation walls, *provided that a final review, and any approval to reuse these native soils is made at the time of construction.*

Excavations should be carried out to conform to the manner specified in Ontario Regulation 213/91 and the Occupational Health and Safety Act and Regulations for Construction Projects (OHSA). All excavations above the water table not exceeding 1.2 m in depth may be constructed with vertical, unsupported slopes. The soils encountered during this investigation are classed by OHSA as Type 3. As such, unsupported walls of excavations in this soil must maintain a gradient of 1 horizontal to 1 vertical (1H:1V) or flatter.

Groundwater infiltration above the water table, if encountered, is expected to be controlled by pumping from collection sumps to an acceptable outlet.

Backfill to foundation walls, both on the exterior and interior, and grade raises below the floor slab should be accomplished using well graded Granular "B" Type 1 material complying with OPSS 1010.

6.3 FOUNDATION DESIGN

It is recommended that all structural loading be supported on spread and continuous strip footings for column and load bearing walls, respectively. The footings should be founded on the undisturbed, compact to dense native sandy soil. For design purposes, it is recommended that footings constructed on this material be proportioned using a maximum allowable bearing pressure of 144 kPa (3,000 psf).

Under no circumstances should the foundations be placed directly on organic materials, loose, frozen subgrade, construction debris, or within ponded water. All exterior footings or footings in unheated areas should be founded at least 1.2 m below the final adjacent grade for frost protection. Footings and walls exposed to frost action should be backfilled with non-frost susceptible granular material.

Prior to forming, all foundation excavations must be inspected and approved by a member of Geo-Logic. This will ensure that the foundation bearing material has been prepared properly at the foundation subgrade level and that the soils exposed are similar to those encountered during this investigation.

The National Building Code (NBC) classes this site as Site Class D, in accordance with clause 4.1.8.4.2-3 and Table 4.1.8.4.A.

For foundations constructed in accordance with the foregoing manner, total and differential settlements are estimated to be less than 25 mm.

6.4 SLAB-ON-GRADE DESIGN

The floor of the proposed building may be constructed as a normal slab-on-grade. The slab should be constructed on granular fill over native, inorganic subsoils. The floor slab should be formed over a base course consisting of at least 150 mm of Granular "A" backfill as per OPSS, compacted to a minimum of 98% of its SPMDD. All grade increases or infilling below the Granular "A" should utilize well graded, free draining Granular "B", Type 1 backfill as per OPSS compacted to a minimum of 98% of its SPMDD. All fill placed as engineered fill must be inspected, approved and compaction verified by personnel from Geo-Logic.

6.5 SEPTIC DESIGN

Based on the results of this assessment, it is our professional opinion that the site is suitable for private septic waste disposal systems. The design of an individual leaching bed should be adjusted to suit local site conditions. It is recommended that the septic system use either a conventional in ground or a fully raised leaching bed. The waste disposal systems should meet Ontario Regulation 22/98 (amendment to Ontario Regulation 403/97 made under the Building Code Act, 1992).

The field and laboratory data indicates that the native soil has an estimated hydraulic conductivity on the order of 10^{-1} to 10^{-3} cm/sec and a related percolation rate (T-time) for design purposes of 2 to 4 min/cm.

6.6 TEST PITS DURING TENDERING

Due to the limitations of this report (see Statement of Limitations, Section 7.0), it is recommended that test pits be dug at representative locations of this site during the tendering phase, with mandatory attendance of interested contractors so that they can make their own assessment regarding earthworks and construction requirements.

6.7 DESIGN REVIEW AND INSPECTION

Due to the preliminary nature of the design details at the time of this report, we recommend that our firm be retained to review the foundation design and grading proposals when they are available.

Geotechnical inspection and review of foundation excavations and compaction procedures must be carried out to ensure compliance with our recommendations.

7.0 STATEMENT OF LIMITATIONS

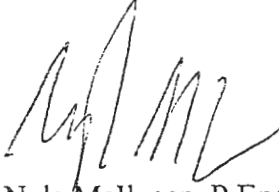
The attached Statement of Limitations is an integral part of this report. Should questions arise regarding any aspect of this report, please contact our office

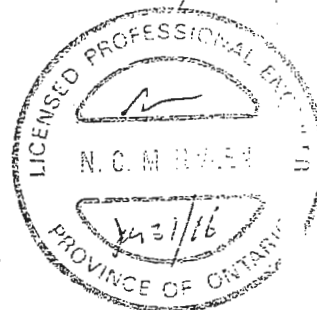
Sincerely yours,

Geo-Logic Inc.
GEOTECHNICAL ENGINEERS
AND HYDROGEOLOGISTS



Garnet Brenchley, P.Eng.
Project Engineer


Nyle McIlveen, P.Eng.
Senior Engineer



/gb

STATEMENT OF LIMITATIONS

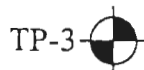
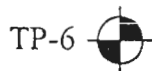
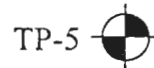
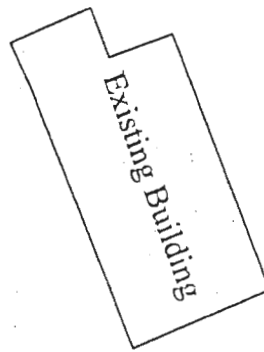
The report is intended for the guidance of the building designers. From a construction standpoint, contractors must make their own assessments of the groundwater and soil conditions at the site and how these will affect their proposed construction methods, techniques and schedules.

The conclusions and recommendations in this report are based on information determined at the test pit locations and on geological data of a general nature which may be available for the area investigated. Soil and groundwater conditions between and beyond the test pits may differ from those encountered at the test pit locations and conditions may become apparent during construction which could not be detected or anticipated at the time of the investigation.

We recommend that we be retained to ensure that all necessary subgrade preparation requirements are met and to confirm that the soil conditions do not deviate materially from those encountered at the test pit locations. In cases where this recommendation is not followed, the company's responsibility is limited to interpreting accurately the information encountered at the test pits.

This report is applicable only to the project described in the introduction, constructed substantially in accordance with details of alignment and elevations quoted in the text. This report has been prepared for the sole use of The Ganaraska Region Conservation Authority. Any use which a third party makes of this report, or any reliance on or decisions to be made based on it, are the responsibility of such third parties. Geo-Logic Inc accepts no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this report.

ENCLOSURES



TEST PIT LOCATION PLAN

GEOTECHNICAL INVESTIGATION
LETTER REPORT
GANARASKA FOREST CENTRE
KIRBY, GANARASKA REGION, ONTARIO

PROJECT NO.: 06-G-184 (G020398-A1)

SCALE: see scale shown

DATE: January, 2007

DRAWN BY: G. Brenchley, P.Eng.

PLATE NO.: 1



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APPENDIX A
TEST PIT LOGS

Log of Test Pit: TP-1

Project No: 06-G-184

Project: Ganaraska Forest Centre

Location: Ganaraska Region, Ontario

GEO-LOGIC INC.
347 Pido Road, Unit 29
PETERBOROUGH, Ontario
K9J 6X7



SUBSURFACE PROFILE				SAMPLE			MOISTURE CONTENT				Piezometer Data	REMARKS
DEPTH	SYMBOL	DESCRIPTION	DEPTH (m)	NUMBER	TYPE	Vapours (ppm)	% dry weight					
							20	40	60	80		
0 ft 0 m		Ground Surface										
		TOPSOIL	0.28									
1		LOAM Reddish loam	0.46									
2		SAND Grey SAND, Some Gravel, Cobbles, Occasional Boulders; Damp, Compact										Open test pit remained dry throughout its excavation
3												
4												
5							1	GS				
6												
7												
8		Test Pit terminated	2.44									

Open test pit remained dry throughout its excavation

Completion Depth (m): 2.4

Excavating Company: McQuaid Contracting

Date of Excavation: December 15, 2006

Excavating Equipment: Back Hoe

Geologist/Technologist: G. Brenchley, P.Eng.

Sampling: Grab Samples

Log of Test Pit: TP-2

Project No: 06-G-184

Project: Ganaraska Forest Centre

Location: Ganaraska Region, Ontario

GEO-LOGIC INC.
347 Pido Road, Unit 29
PETERBOROUGH, Ontario
K9J 6X7



SUBSURFACE PROFILE				SAMPLE			MOISTURE CONTENT					Piezometer Data	REMARKS
DEPTH	SYMBOL	DESCRIPTION	DEPTH (m)	NUMBER	TYPE	Vapours (ppm)	% dry weight						
							20	40	60	80			
0	0	Ground Surface											
		TOPSOIL	0.20										
1		LOAM Redish Brown LOAM, Some Sand Silt and Roots, Damp, Loose	0.48										
2		SAND Grey SAND, Some Gravel, Cobbles, Damp, Compact											
3	1												
4		Boulders present	1.22										
5													
6													
7	2												
8													
9													
10	3	Test Pit terminated	3.05										

Open test pit remained dry
throughout its excavation

Completion Depth (m): 3.0

Excavating Company: McQuaid Contracting

Date of Excavation: December 15, 2006

Excavating Equipment: Back Hoe

Geologist/Technologist: G. Brenchley, P.Eng.

Sampling: Grab Samples

Log of Test Pit: TP-3

Project No: 06-G-184

Project: Ganaraska Forest Centre

Location: Ganaraska Region, Ontario

GEO-LOGIC INC.
347 Pido Road, Unit 29
PETERBOROUGH, Ontario
K9J 6X7



SUBSURFACE PROFILE				SAMPLE			MOISTURE CONTENT				Piezometer Data	REMARKS
DEPTH	SYMBOL	DESCRIPTION	DEPTH (m)	NUMBER	TYPE	Vapours (ppm)	% dry weight					
							20	40	60	80		
ft 0		Ground Surface										
m 0		TOPSOIL	0.18									
		LOAM Reddish Brown LOAM, Some Sand Silt and Roots, Damp, Loose	0.28									
1		SAND Grey SAND, Some Gravel, Damp, Compact										
2												
3												
4												
5												
6												
7												
8			2.44									
		Test Pit terminated										

Open test pit remained dry throughout its excavation

Open test pit remained dry throughout its excavation

Completion Depth (m): 2.4

Excavating Company: McQuaid Contracting

Date of Excavation: December 15, 2006

Excavating Equipment: Back Hoe

Geologist/Technologist: G. Brenchley, P.Eng.

Sampling: Grab Samples

Log of Test Pit: TP-4

Project No: 06-G-184

Project: Ganaraska Forest Centre

Location: Ganaraska Region, Ontario

GEO-LOGIC INC.
347 Pido Road, Unit 29
PETERBOROUGH, Ontario
K9J 6X7



SUBSURFACE PROFILE				SAMPLE			MOISTURE CONTENT					Piezometer Data	REMARKS
DEPTH	SYMBOL	DESCRIPTION	DEPTH (m)	NUMBER	TYPE	Vapours (ppm)	% dry weight						
							20	40	60	80			
0	ft	Ground Surface											
0	m	TOPSOIL											
			0.28										
1		LOAM											
		Redish Brown LOAM, Some Roots	0.41										
		SAND											
		Grey SAND, Some Gravel and Cobbles, Occasional Boulders, Damp, Compact											
2													
3													
4													
5				1	GS	•							
6													
7													
8													
9													
10			3.05										
		Test Pit terminated											

Open test pit remained dry throughout its excavation

Completion Depth (m): 3.0

Excavating Company: McQuaid Contracting

Date of Excavation: December 15, 2006

Excavating Equipment: Back Hoe

Geologist/Technologist: G. Brenchley, P.Eng.

Sampling: Grab Samples

Log of Test Pit: TP-5

Project No: 06-G-184

Project: Ganaraska Forest Centre

Location: Ganaraska Region, Ontario

GEO-LOGIC INC.
347 Pido Road, Unit 29
PETERBOROUGH, Ontario
K9J 6X7



SUBSURFACE PROFILE				SAMPLE			MOISTURE CONTENT				Piezometer Data	REMARKS		
DEPTH	SYMBOL	DESCRIPTION	DEPTH (m)	NUMBER	TYPE	Vapours (ppm)	% dry weight							
							20	40	60	80				
ft m		Ground Surface												
0 0		TOPSOIL	0.20											
1		LOAM Redish Brown LOAM	0.41											
2		SAND Grey SAND, Some Gravel and Cobbles, Trace Silt, Damp, Compact												
3														
4														
5														
6							1	GS	•					
7														
8														
9														
10														
3			3.05											
		Test Pit terminated												

Open test pit remained dry throughout its excavation

Completion Depth (m): 3.0

Excavating Company: McQuaid Contracting

Date of Excavation: December 15, 2006

Excavating Equipment: Back Hoe

Geologist/Technologist: G. Brenchley, P.Eng.

Sampling: Grab Samples

Log of Test Pit: TP-6

Project No: 06-G-184

Project: Ganaraska Forest Centre

Location: Ganaraska Region, Ontario

GEO-LOGIC INC.
347 Pido Road, Unit 29
PETERBOROUGH, Ontario
K9J 6X7



SUBSURFACE PROFILE				SAMPLE			MOISTURE CONTENT				Piezometer Data	REMARKS
DEPTH	SYMBOL	DESCRIPTION	DEPTH (m)	NUMBER	TYPE	Vapours (ppm)	% dry weight					
							20	40	60	80		
0 ft 0 m		Ground Surface	0.00									
		TOPSOIL	0.18									
1		SAND Dark Brown SAND, Some Gravel and Cobbles, Damp										
2			0.79									
3		SILTY SAND Brown / dark brown layered Silty Sand, uniformly-graded, damp, compact										
4												
5												
6				1	GS							
7												
8			2.44									
		Test Pit terminated										

Completion Depth (m): 2.4

Excavating Company: McQuaid Contracting

Date of Excavation: December 15, 2006

Excavating Equipment: Back Hoe

Geologist/Technologist: G. Brenchley, P.Eng.

Sampling: Grab Samples

Log of Test Pit: TP-7

Project No: 06-G-184

Project: Ganaraska Forest Centre

Location: Ganaraska Region, Ontario

GEO-LOGIC INC.
347 Pido Road, Unit 29
PETERBOROUGH, Ontario
K9J 6X7



SUBSURFACE PROFILE				SAMPLE			MOISTURE CONTENT				Piezometer Data	REMARKS
DEPTH	SYMBOL	DESCRIPTION	DEPTH (m)	NUMBER	TYPE	Vapours (ppm)	% dry weight					
							20	40	60	80		
ft m		Ground Surface	0.00									
0		TOPSOIL	0.18									
1		LOAM Reddish Loam, Damp	0.46									
2		SAND Grey Gravelly SAND, Some Cobbles, Damp, Dense	1.22									
3												
4		GRAVEL Brown Gravel with Sand, coarse-grained, damp, compact to dense	1.83	1	GS							
5												
6		SAND Brown Sand, layered, damp, compact										
7												
8												
9												
10			3.05									
		Test Pit terminated										

Open test pit remained dry throughout its excavation

Completion Depth (m): 3.0

Excavating Company: McQuaid Contracting

Date of Excavation: December 15, 2006

Excavating Equipment: Back Hoe

Geologist/Technologist: G. Brenchley, P.Eng.

Sampling: Grab Samples

Log of Test Pit: TP-8

Project No: 06-G-184

Project: Ganaraska Forest Centre

Location: Ganaraska Region, Ontario

GEO-LOGIC INC.
347 Pido Road, Unit 29
PETERBOROUGH, Ontario
K9J 6X7



SUBSURFACE PROFILE				SAMPLE			MOISTURE CONTENT				Piezometer Data	REMARKS
DEPTH	SYMBOL	DESCRIPTION	DEPTH (m)	NUMBER	TYPE	Vapours (ppm)	% dry weight					
							20	40	60	80		
ft	m											
0	0	Ground Surface	0.00									
		TOPSOIL										
			0.23									
1		LOAM Reddish Brown LOAM, Damp										
			0.51									
2		SAND Brown/ Light Brown Layered SAND, Some Gravel and Cobbles, Damp, Dense										
3	1											
4												
5												
6												
7	2											
8			2.44									
		Test Pit terminated										

Open test pit remained dry throughout its excavation

Completion Depth (m): 2.4

Excavating Company: McQuaid Contracting

Date of Excavation: December 15, 2006

Excavating Equipment: Back Hoe

Geologist/Technologist: G. Brenchley, P.Eng.

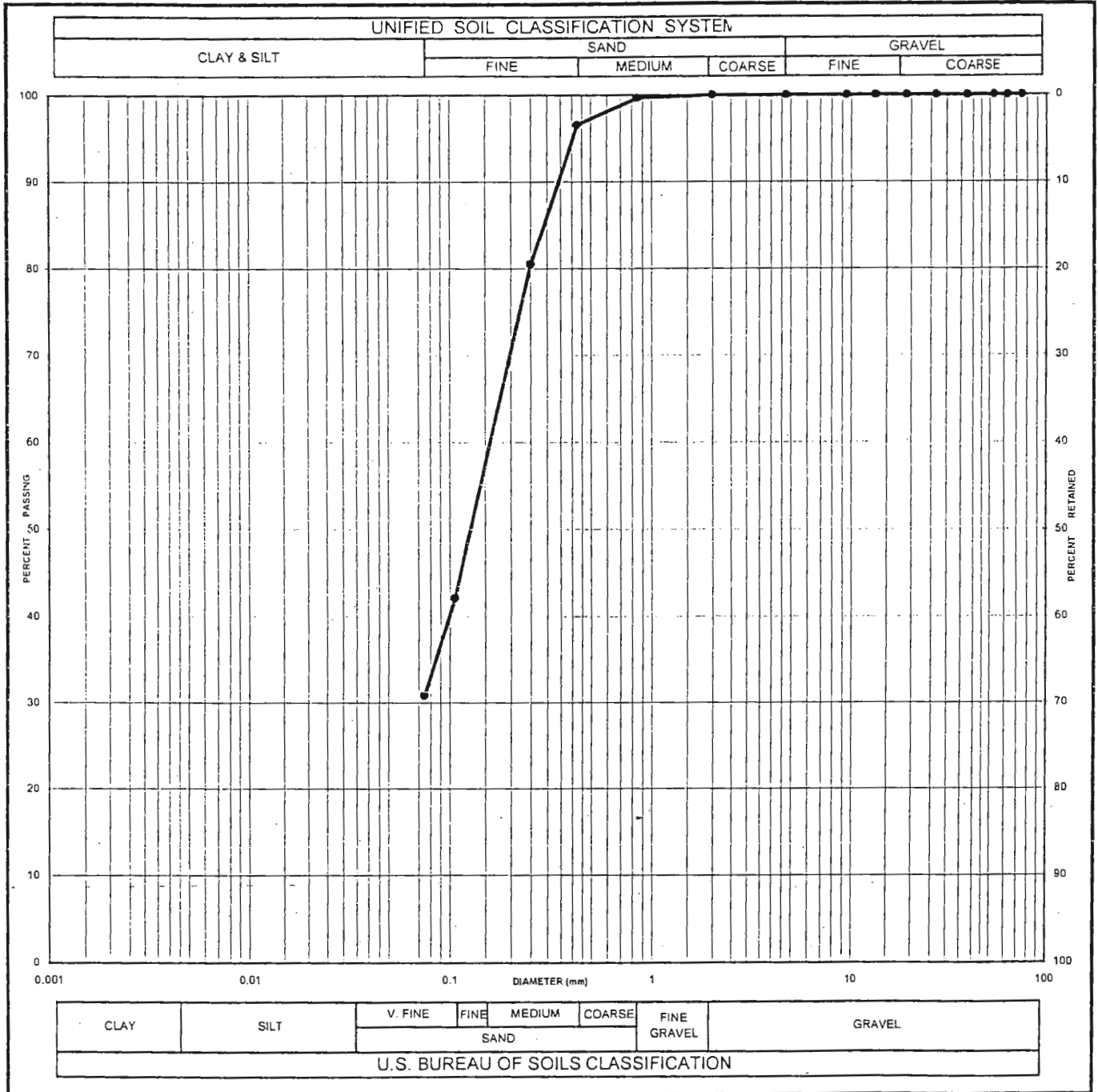
Sampling: Grab Samples

APPENDIX B

LABORATORY DATA

GRAIN SIZE DISTRIBUTION DATA

PROJECT NO.: 06-G-184 PLATE NO.: B-1
 CLIENT: GRCA DATE OF TESTS: 4-Jan-07
 PROJECT: GRCA Forest Centre - Kirby

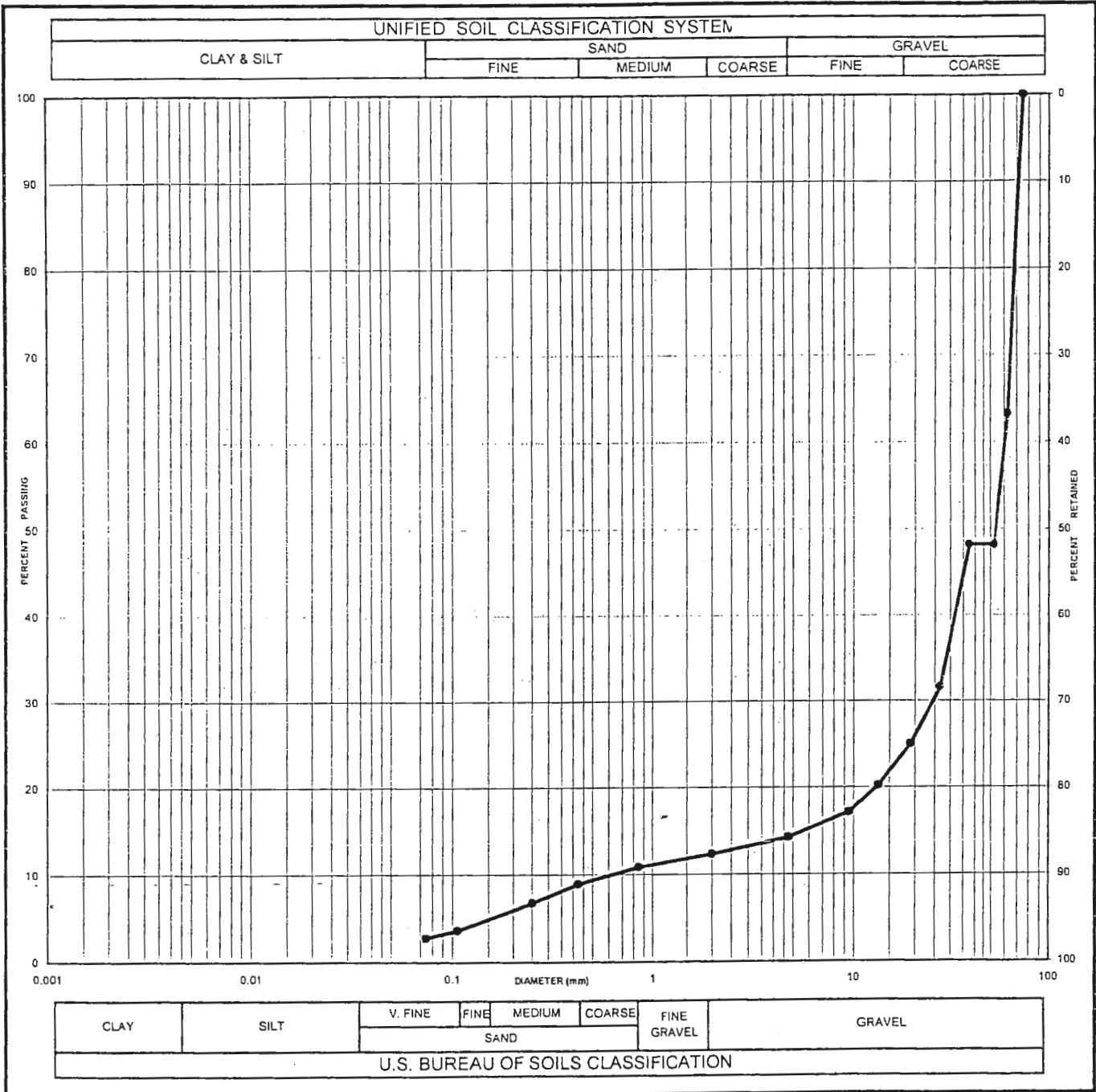


SAMPLE No.	DEPTH (ELEVATION)	DESCRIPTION	Gravel	Sand	Silt	Clay	CLASS.
TP-6, GS-1	1.50 - 2.10m	SILTY SAND	0	69	31		SM

NOTE: UNIFIED SOIL CLASSIFICATION SYSTEM ("USCS")

GRAIN SIZE DISTRIBUTION DATA

PROJECT NO.: 06-G-184 PLATE NO.: B-2
 CLIENT: GRCA DATE OF TESTS: 4-Jan-07
 PROJECT: GRCA Forest Centre - Kirby



SAMPLE No.	DEPTH (ELEVATION)	DESCRIPTION	Gravel	Sand	Silt	Clay	CLASS.
TP-7, GS-1	1.20 - 1.50m	GRAVEL with SAND	86	11	3		GP

NOTE: UNIFIED SOIL CLASSIFICATION SYSTEM ("USCS")

